#### Getting Familiar with the BCOA-ME Design Guide

Bonded Concrete Overlay of Asphalt Pavements Mechanistic-Empirical Design Guide (BCOA – ME)





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#### EFFECT OF FIBER ON JOINT PERFORMANCE



#### Benefits of structural fibers

Current: Increase MOR

#### Advantage

- Increase fracture toughness
- Decrease crack/joint width
- Potential increase in load transfer

#### Disadvantage

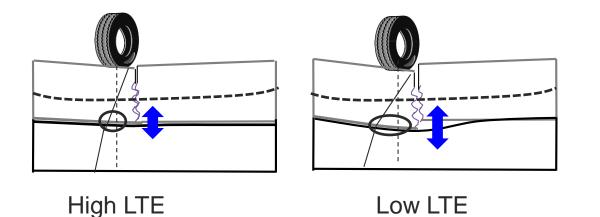
Increase cost of PCC by approx. 20%





#### Influence on load transfer

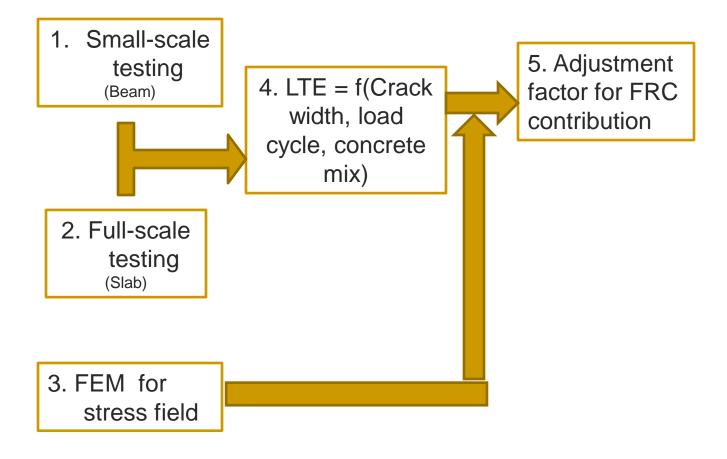
- 1. Reduce load related stress on the loaded slab;
- 2. Reduce debonding tensile "stresses between the layers.



 $LTE = \frac{\Delta u}{\Delta_L} \times 100 \text{ percent}$  $\Delta_U \text{ and } \Delta_L = \text{def. at}$ unloaded and loaded slabs.

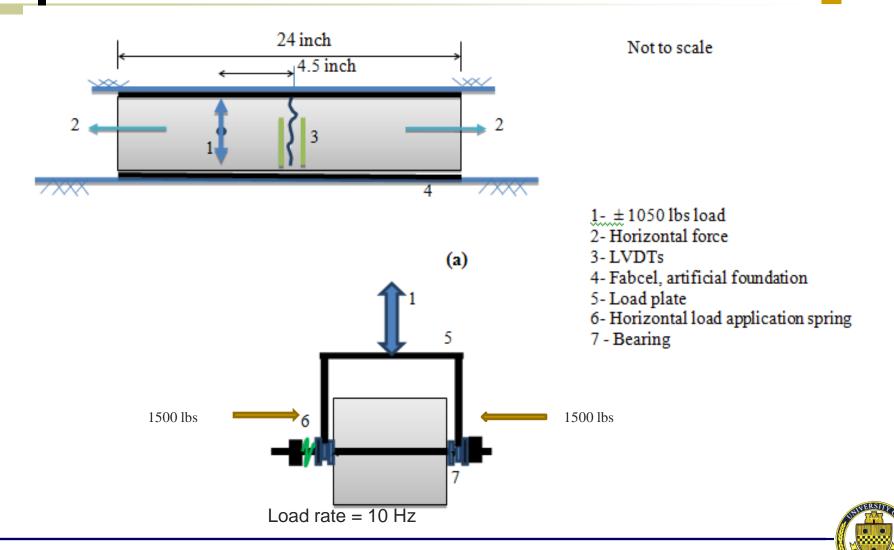


# Strategy

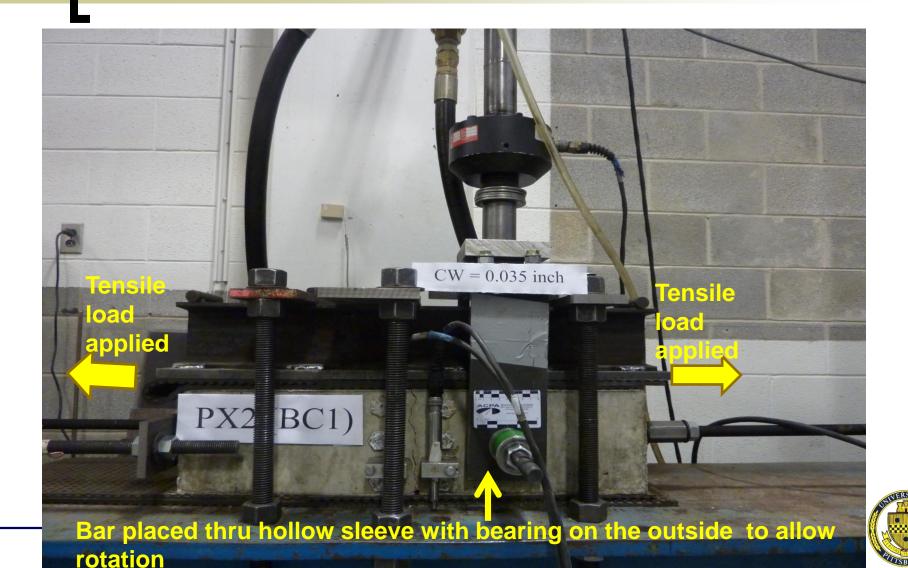




#### 1. Development of small-scale test



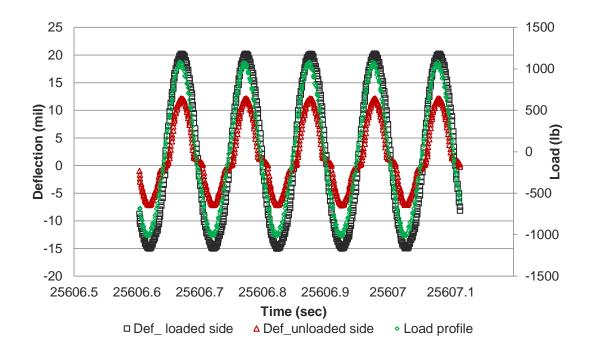
#### 1. Small-scale LTE testing



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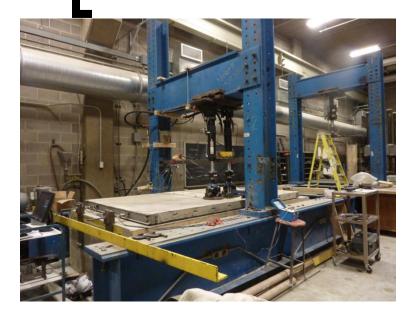








#### 2. Large-scale testing







# Fibers Considered

Synthetic Fiber	Brand	Length (inch)	Shape	Cross section (inch x inch)	Specific gravity	Aspect ratio	Quantity (lb/cy)
Straight	Strux: 90/40	1.57	Rectangular	0.05 x 0.004	0.92	90	5.25
Crimped	Enduro 600	1.75	Rectangular	0.05 x 0.03	0.91	40	6.2



Strux 90/40 (F1)

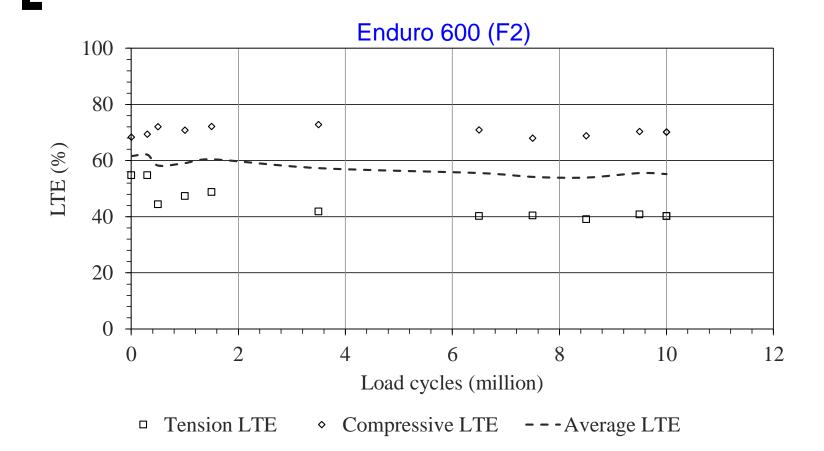
Target residual strength = 20%

Enduro 600 (F2)



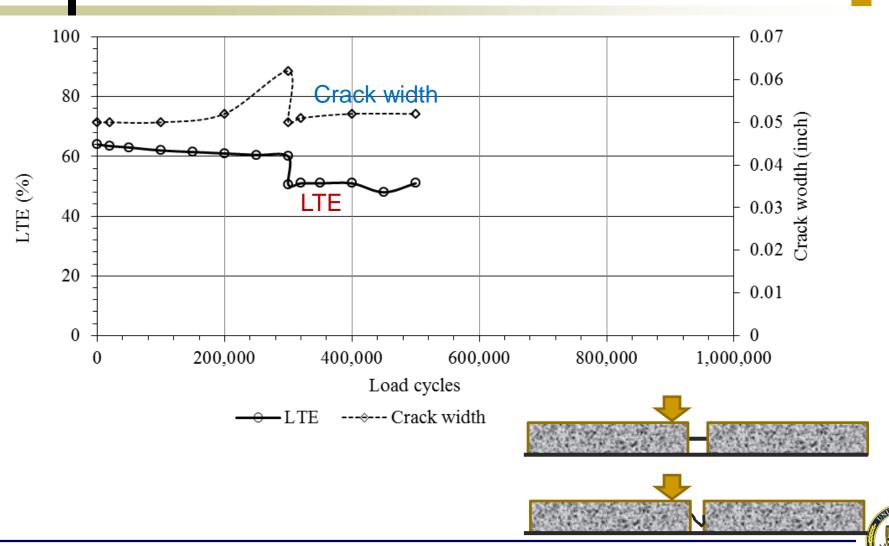


#### No Fatigue of Fiber

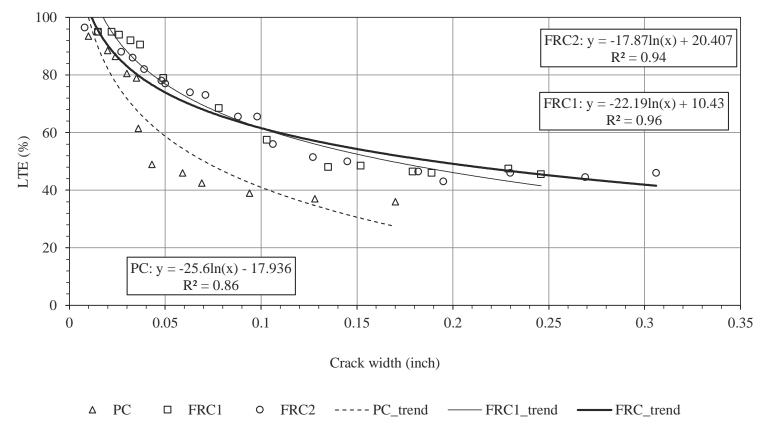




# Effects of reduction in crack width

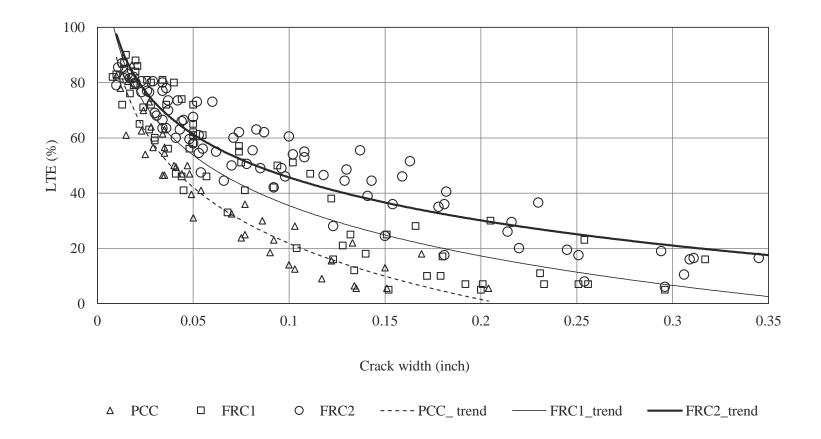


## Slab performance





#### Beam performance





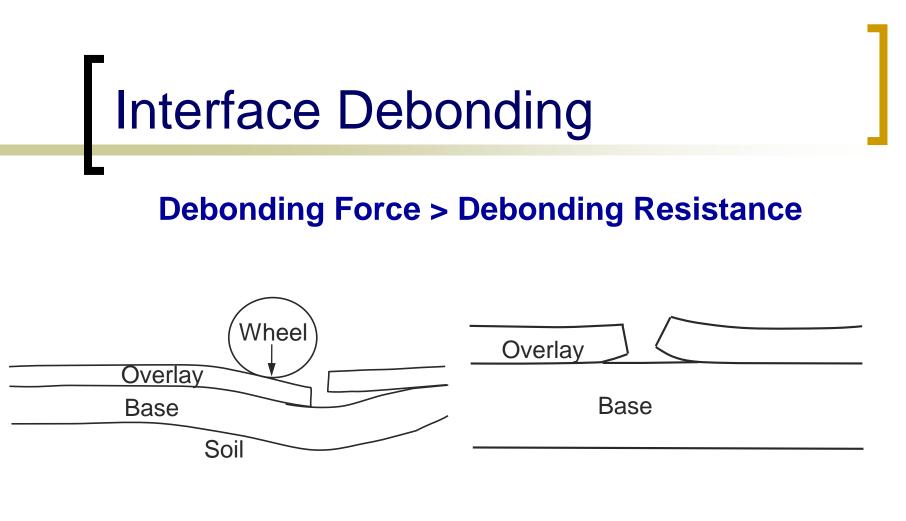
#### Comments

- Even though the shapes, sizes and aspect ratios of the two fibers were different, performances were similar.
- Residual strength fiber selection criteria could possibly indicate equivalent joint performance.
- Fibers increases LTE by 10%
- Fiber did not exhibit fatigue after 10 million load applications
- Effectiveness of fiber appears to decrease when crack width is less than max. crack opening experienced



#### DEBONDING MODEL





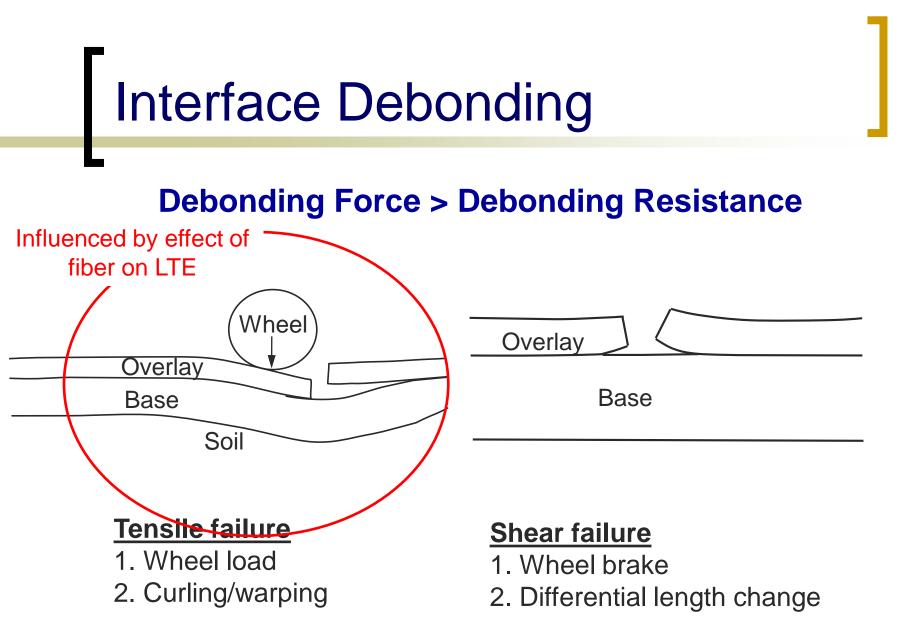
#### **Tensile failure**

- 1. Wheel load
- 2. Curling/warping

#### Shear failure

- 1. Wheel brake
- 2. Differential length change

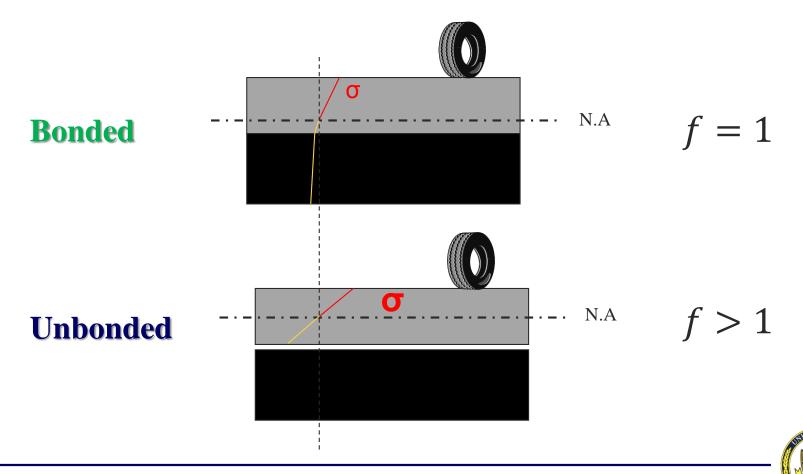




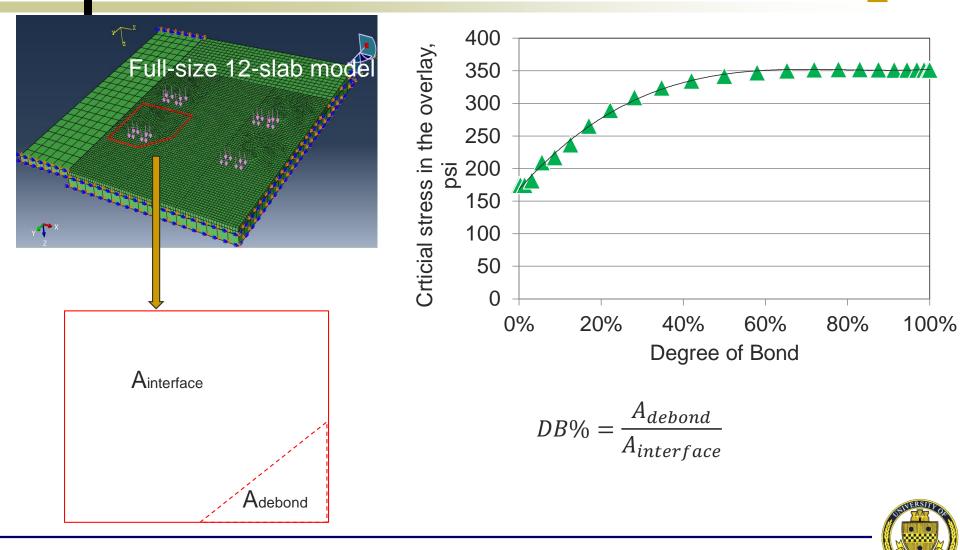


#### Interface bond

 $\sigma_{design} = f(degree \ of \ bond) \cdot \sigma_{bonded}$ 



## Effect of partial bonding



# Considerations in current design

	Degree of Bond (DB)		
CDOT	Increase stress by 65%-59% <sup>1</sup>		
NJDOT	Engineering judgment		
PCA	Increase stress by 36% <sup>2</sup>		
ICT	Same as PCA method		

1. Based on Colorado data, 2. based on Missouri and Colorado Data



# Proposed debonding model

Paris' law:

$$A_{debond} = c \left(\frac{\Delta G}{Gc}\right)^m N$$

 $\Delta G = Energy release rate, a function of appled load$ 

Gc= Critical energy release rate, a function of material testing

c,m= Coefficients from slab testing

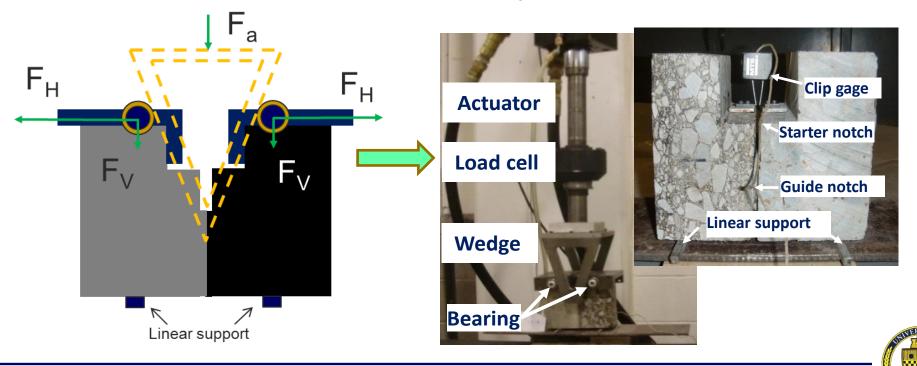
N= Number of loads



#### **Material Characterization**

$$A_{debond} = c \left(\frac{\Delta G}{Gc}\right)^m N$$

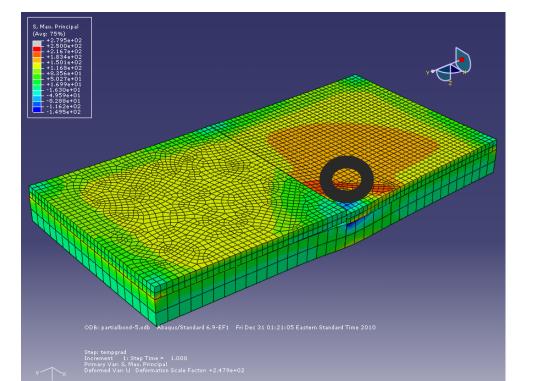
 $logGc = B_0 + B_1 logR + B_2 loga_0$ R: Interface roughness  $a_0$ : Initial flaw size



#### Calculation of debonding force

$$A_{debond} = c \left(\frac{\Delta G}{Gc}\right)^m N$$

$$\Delta G = f(L, h_{pcc}, h_{HMA}, E_{HMA}, P, A_{debond})$$



L: Slab size

*h*<sub>pcc</sub>: Overlay thickness

 $h_{HMA}$ : Asphalt thickness

E<sub>HMA</sub>: Asphalt stiffness

P:load vector

A<sub>debond</sub>: current debonding size



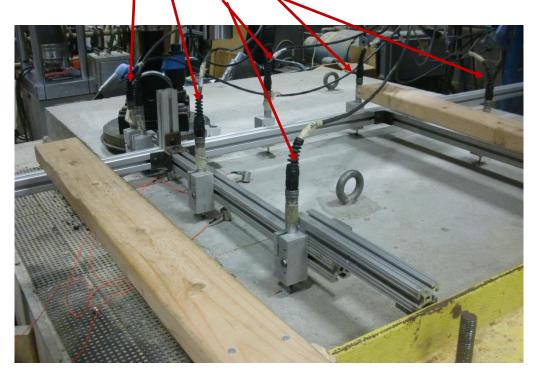
# Calibration of the fatigue law $A_{debond} = c \left(\frac{\Delta G}{Gc}\right)^m N$



#### Methods to determine A<sub>debond</sub> @ N

#### 1. Deflection method

 $A_{debond} \neq c \left(\frac{\Delta G}{Gc}\right)^m N$ 



Deflections measured during testing of slabs



Deflections from FE models w/ various DB%



